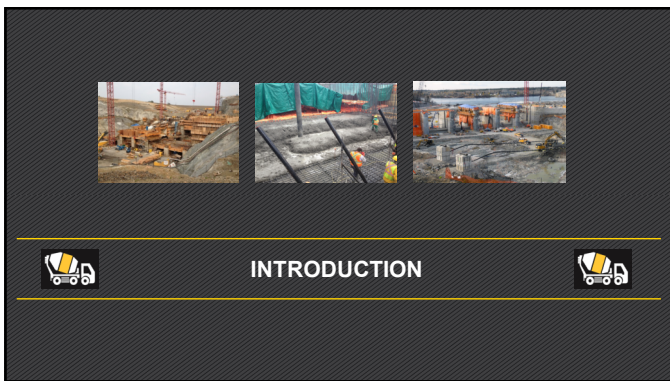




1



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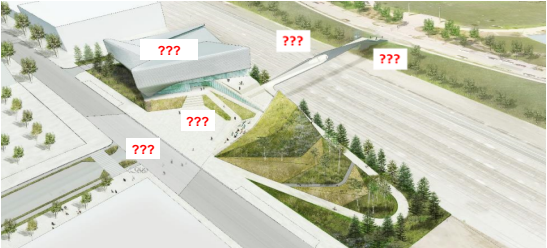
MY POINT OF VIEW



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WHY CONSTRUCTABILITY IS IMPORTANT?




What would be the conditions and applicable Standards, Specs and Codes for concrete at each area identified above? Same, or not? Why?

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IMPROVE CONSTRUCTABILITY = IMPROVED PRODUCTIVITY

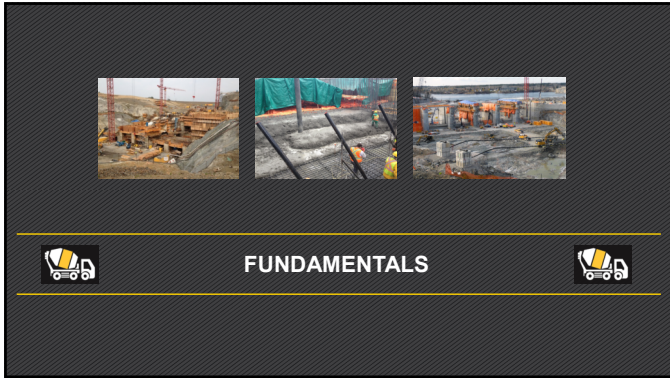


Accelerated Schedule / Fast-Track

Emergencies



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TYPES OF MASS CONCRETE

- Traditional mass concrete:** Use for structures such as dams, typically lightly reinforced, where the majority of the structure is mostly mass concrete and is constructed of sequential placements.
 
- Thermally-controlled concrete:** Use for structures such as high-rise building foundations or bridges, typically heavily reinforced, where the majority of the structures are individual placements.
 

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MASS CONCRETE – IN SIMPLE TERMS...

Equivalent Concrete Depth, ft/m	Placement Thickness (Minimum Diameter), ft																	
	1/2	1	1 1/2	2	2 1/2	3	3 1/2	4	4 1/2	5	5 1/2	6	6 1/2	7	8	8 1/2	9	9 1/2
200	+ Less Risk												+ More Risk					
300	+ Less Risk												+ More Risk					
400	+ Less Risk												+ More Risk					
500	+ Less Risk												+ More Risk					
600	+ Less Risk												+ More Risk					
700	+ Less Risk												+ More Risk					
800	+ Less Risk												+ More Risk					
900	+ Less Risk												+ More Risk					
1000	+ Less Risk												+ More Risk					

Hotter (at 1000 ft depth) **Larger** (at 10 ft placement thickness)

• Thermal risks increase with higher cementitious materials content and larger structures since these may show mass concrete behavior.

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CONCRETE MIXTURES

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CONCRETE MIXTURE DEVELOPMENT




BOOK-CRETE (30 TO 60 days)	LAB-CRETE (90 TO 120 days)	CON-CRETE (15 TO 60 days)
<ul style="list-style-type: none"> • Technical requirements • Materials availability • Procurement • Pre-qualification • Transportation 	<ul style="list-style-type: none"> • Lab resources • Trial testing • Materials properties • Fresh/Hardened • Validations 	<ul style="list-style-type: none"> • Batch plant systems • Mockups • Workability/Pumping • Thermal • Optimizations

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CONCRETE MIXTURE PROPORTIONING

STRENGTH	DURABILITY	WORKABILITY	THERMAL	ECONOMICS
<ul style="list-style-type: none"> • w/cm ratio 	<ul style="list-style-type: none"> • Freeze/Thaw • Sulfate Resistance • Permeability • Service Life 	<ul style="list-style-type: none"> • Slump Retention • Consolidation • Finishing 	<ul style="list-style-type: none"> • Tmax / Tdiff • Thermal Control 	<ul style="list-style-type: none"> • Mix Costs • Field Operations • Quality

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CONCRETE MIXTURE OPTIMIZATION

- Aggregate grading makes a difference
- Admixtures enhance performance

The slide contains two main sections. On the left, under 'Aggregate grading makes a difference', there is a graph showing cumulative percentage passing versus sieve size for three different aggregate gradations: 'Normal', 'Lean', and 'Rich'. Below the graph are three diagrams of concrete cross-sections showing different aggregate distributions. On the right, under 'Admixtures enhance performance', there are four images: a row of four test tubes showing different colored liquids, a diagram of a concrete particle with admixtures, a person in a hard hat, and a person holding a test tube.

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TEMPERATURE LIMITS

The slide features three photographs of construction sites. The first shows a large concrete slab being poured. The second shows a worker on a site with rebar. The third shows a completed concrete slab. At the bottom, there are two icons of a truck and a shovel.

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ADIABATIC TEMPERATURE RISE (ATR)

- ATR is mainly affected by type and quantity of cementitious materials
- Reducing ATR
 - Reduce total cementitious materials
 - Use less Portland Cement
 - Use more low heat SCMs (Class F fly ash and slag)

Description		
Very low	Traditional Mass Concrete	ATR < 80 °F
Low		
Moderate	Thermally Controlled Concrete	ATR > 80°F
High		
Very high		

The slide includes a graph showing 'Maximum Temperature Rise (°F)' on the y-axis and 'Time (Hours)' on the x-axis. It plots curves for 'Normal concrete', 'Lean concrete', and 'Rich concrete'. Below the graph is a 3D diagram of a concrete structure with labels for 'Reinforcing Steel', 'Fly Ash (C)', 'Class F Fly Ash', and 'Calcined Slag'.

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ADIABATIC TEMPERATURE RISE (ATR)

Methods	Description
Simplified approach	Use a mathematical expression such as Gajda et al, 2014: $Rise = 0.16 * (1.0 * Cem + 0.5 * FAsh + 0.8 * CAsh + 1.2 * SFMK + Factor * Slag), F$
Computer modeling	Use a simplified analytical/modeling software such as ConcreteWorks [®] ; an advanced finite element analytical/modeling software; or a combination thereof.
Semi-adiabatic testing	Use a commercially available semi-adiabatic test chamber; a field fabricated super-insulated test block; a lab fabricated insulated box; or a combination thereof.
Adiabatic testing	Use adiabatic test method such as CRD-C 38, Method of Test for Temperature Rise in Concrete
Field placement	Use a field mass placement having large dimensions, such as a crane footing, with insulation and monitoring temperature at locations intended to be monitored during construction
Other methods	Use a method described in ACI 207.2R, such as Schmidt's method; or other method approved by the owner

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ADIABATIC TEMPERATURE RISE (ATR)

The diagram shows: **Semi-Adiabatic (Test Block) 65°F** + **Computer Modeling** = **Full-Adiabatic (Calculated) 82°F**. The graph plots temperature (°F) against time (hours), showing a rapid initial rise followed by a plateau. The fresh concrete placing temperature is indicated at the start of the curve.

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FRESH CONCRETE PLACING TEMPERATURE

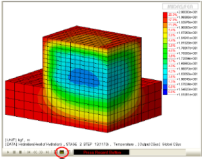
- Main concerns**
 - ✓ Effect on fresh properties
 - ✓ Effect on thermal and shrinkage
 - ✓ Effect on ultimate strength and long-term durability
- Industry limits**
 - ✓ Typical limit (ACI 301/305/306)
 - 95 °F max., maximum
 - 40 to 50 °F, minimum (depending on placement attributes)

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MAXIMUM CONCRETE TEMPERATURE

- Main concerns**
 - ✓ Long term effect on strength and durability
- Industry limits**
 - ✓ Typical "safe" limit (ACI 301)
 - 158/160 °F – with / without mitigation
 - ✓ Alternative "safe" limit (ACI 308 / Z01 / CSA A23.1)
 - 158/160 °F – without mitigation
 - Up to 185 °F – with mitigation (add SCMs)
 - Greater than 185 °F – not permitted (may be justified with testing)
 - ✓ Increased acceptance of alternative limits but not yet a global consensus



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ALTERNATIVE MAXIMUM CONCRETE TEMPERATURE

Table 6.2.2.2—Recommended measures for reducing potential for DEF in concrete exposed to elevated temperatures at early ages*

Maximum concrete temperature T	Prevention required
T ≤ 158°F (70°C)	No prevention required
158°F < T ≤ 185°F (70°C < T ≤ 85°C)	Use one of the following approaches to minimize the risk of expansion: <ol style="list-style-type: none"> 1. Portland cement meeting requirements of ASTM C150/C150M moderate or high sulfate-resisting and low alkali cement with a fineness value less than or equal to 430 m²/kg 2. Portland cement with a 1-day mortar strength (ASTM C109/C109M) less than or equal to 2850 psi (20 MPa) 3. Any ASTM C150/C150M portland cement in combination with the following proportions of pozzolan or slag cement: <ol style="list-style-type: none"> a) Greater than or equal to 25 percent fly ash meeting the requirements of ASTM C618 for Class F fly ash b) Greater than or equal to 15 percent fly ash meeting the requirements of ASTM C618 for Class C fly ash c) Greater than or equal to 10 percent slag cement meeting the requirements of ASTM C98/C98M d) Greater than or equal to 5 percent silica fume (meeting ASTM C1240) in combination with at least 25 percent slag cement e) Greater than or equal to 5 percent silica fume (meeting ASTM C1240) in combination with at least 20 percent Class F fly ash f) Greater than or equal to 10 percent metakaolin meeting ASTM C618 4. An ASTM C99/C99M or ASTM C1157/C1157M blended hydraulic cement with the same pozzolan or slag cement content as listed in Item 3.
T > 185°F (85°C)	The internal concrete temperature should not exceed 185°F (85°C) under any circumstances.

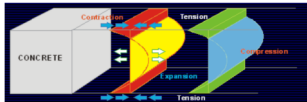
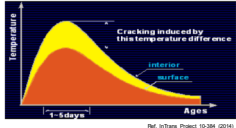
*Assembled from Ghosh et al. (1980), Ramlachan et al. (2003), Thomas (2001), Thomas et al. (2008).

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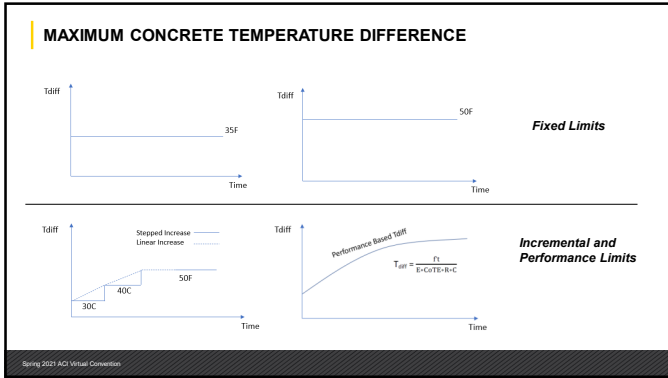
MAXIMUM CONCRETE TEMPERATURE DIFFERENCE

- Main concerns**
 - ✓ Thermal cracking
- Industry limits**
 - ✓ Typical "safe" limit (ACI 301)
 - 35 °F fixed limit
 - ✓ Alternative "safe" limit (CSA A23.1)
 - Higher fixed limit (low CoTE concrete)
 - Incremental limit
 - Performance based with testing
 - ✓ Increased acceptance of alternative limits but not yet a global consensus

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- ### WHAT IS A THERMAL CONTROL PLAN?
- TOOL for the Contractor to plan, manage and execute mass placements:
 - ✓ Control & monitor temperature
 - ✓ Reduce thermal shrinkage
 - ✓ Minimize thermal cracks
 - Know the behavior, challenges and effects:
 - ✓ Mixture proportions
 - ✓ Placing cycles, heat sinks, formwork or insulation removal
 - ✓ Maximum temp and temp difference
 - ✓ Duration of thermal control and monitoring
 - ✓ Corrective measures (e.g. pre-cooling or post-cooling)
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IMPLEMENTING A THERMAL CONTROL PLAN

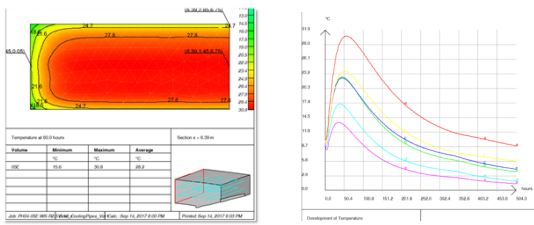
1. Analytical Modeling
2. Thermal Control Plan
3. Field Implementation
4. Verification and Validation
5. Revise and Update



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ANALYTICAL MODELING



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THERMAL CONTROL PLAN

Thermal Control Table
Block Product of
4500 PSI | 65% Grade 200 Gr#

Ambient Temp ¹	Concrete Temp ²	No Insulation			Light Insulation ³			Medium Insulation ⁴			Heavy Insulation ⁵		
		Max Core Temp	Max Thermal Control ⁶	End of Day	Max Core Temp	Max Thermal Control ⁶	End of Day	Max Core Temp	Max Thermal Control ⁶	End of Day	Max Core Temp	Max Thermal Control ⁶	End of Day
90	90	150	90	N/A	100	30	5.5	171	25	7.5	179	16	8.5
	85	145	85	N/A	105	32	5.0	165	22	7.0	168	14	8.5
	80	140	80	N/A	110	35	4.5	160	20	6.5	163	12	8.5
70	90	130	90	N/A	100	30	5.5	161	25	8.0	161	18	12.0
	85	125	85	N/A	105	32	5.0	155	23	7.5	155	16	12.0
	80	120	80	N/A	110	35	4.5	150	21	7.0	150	14	12.0
50	90	120	90	N/A	100	30	5.5	149	24	8.0	149	19	14.0
	85	115	85	N/A	105	32	5.0	143	22	7.5	143	17	14.0
	80	110	80	N/A	110	35	4.5	138	20	7.0	138	15	14.0
30 ⁷	90	100	90	N/A	90	20	6.0	131	31	9.0	128	21	16.0
	85	95	85	N/A	95	24	5.5	125	29	8.5	125	19	16.0
	80	90	80	N/A	100	28	5.0	120	26	8.0	120	17	16.0

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FIELD IMPLEMENTATION

1. Temperature Monitoring
2. Pre-Cooling
3. Insulation
4. Post-Cooling

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VERIFICATION / VALIDATION

Time	Location	Temperature (°C)	Time	Location	Temperature (°C)
0	1	15.0	0	1	15.0
1	1	15.0	1	1	15.0
2	1	15.0	2	1	15.0
3	1	15.0	3	1	15.0
4	1	15.0	4	1	15.0
5	1	15.0	5	1	15.0
6	1	15.0	6	1	15.0
7	1	15.0	7	1	15.0
8	1	15.0	8	1	15.0
9	1	15.0	9	1	15.0
10	1	15.0	10	1	15.0
11	1	15.0	11	1	15.0
12	1	15.0	12	1	15.0
13	1	15.0	13	1	15.0
14	1	15.0	14	1	15.0
15	1	15.0	15	1	15.0
16	1	15.0	16	1	15.0
17	1	15.0	17	1	15.0
18	1	15.0	18	1	15.0
19	1	15.0	19	1	15.0
20	1	15.0	20	1	15.0

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REVISE / UPDATE

- When a revision or update may be necessary:
 - ✓ Field conditions are known, but the model is inaccurate and not conservative
 - ✓ Field conditions are known, but the model is inaccurate and highly conservative
 - ✓ Proposed change to construction sequence, materials, means/methods, etc.
- Other things to consider when evaluating a revision or update:
 - ✓ Inadequate field implementation
 - ✓ Different field conditions
 - ✓ Concrete mix changes
 - ✓ Formwork or insulation changes
 - ✓ Unknown field operations change

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CSA A23.1-19, CLAUSE 7.6.3 – MASS CONCRETE

7.6.3.2 Temperature requirements for mass concrete

7.6.3.2.1 General
The temperature requirements for mass concrete placements shall comply with Clauses 7.6.3.2.2 to 7.6.3.2.5 unless otherwise specified or approved by the owner.

7.6.3.2.2 Adiabatic Temperature Rise

7.6.3.2.3 Maximum Placing Concrete Temperature

7.6.3.2.4 Maximum Concrete Temperature

7.6.3.2.5 Maximum Concrete Temperature Difference

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CSA A23.1-19, CLAUSE 7.6.3 – MASS CONCRETE

7.6.3.2.2 Adiabatic temperature rise

The predicted adiabatic temperature rise of concrete mixes for mass placements shall be reported in the concrete mix design submittal. The method to calculate adiabatic temperature rise shall be identified and reviewed by the concrete producer and contractor.

Notes:

- 1) Adiabatic temperature rise of mass concrete can be determined by using tests and analytical methods. Refer to information in Annex T for guidance.
- 2) Refer to Clause 6 of CSA A23.2-24C for further information regarding mix design submittals.

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CSA A23.1-19, CLAUSE 7.6.3 – MASS CONCRETE

7.6.3.2 Temperature requirements for mass concrete

7.6.3.2.1 General
The temperature requirements for mass concrete placements shall comply with Clauses 7.6.3.2.2 to 7.6.3.2.5 unless otherwise specified or approved by the owner.

7.6.3.2.2 Adiabatic Temperature Rise

7.6.3.2.3 Maximum Placing Concrete Temperature

7.6.3.2.4 Maximum Concrete Temperature

7.6.3.2.5 Maximum Concrete Temperature Difference

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CSA A23.1-19, CLAUSE 7.6.3 – MASS CONCRETE

7.6.3.2.3 Maximum placing concrete temperature
In the absence of a thermal control plan, the maximum placing concrete temperature for mass concrete placements shall comply with Table 14 for permissible concrete temperatures at placing.
Note: Lowering the placing concrete temperature is beneficial to reduce temperature rise of mass concrete that can lead to high concrete temperatures and large temperature differentials.

Table 14
Permissible concrete temperatures at placing
(See Clauses 5.2.5.4.1, 7.2.2.1, 7.5.1.3, 7.6.3.2.3, and 6.5.5.)

Thickness of section, m	Temperatures, °C	
	Minimum	Maximum
< 0.3	10	32
≥ 0.3 – < 1	10	30
≥ 1 – < 2	5	25
≥ 2	5	20

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CSA A23.1-19, CLAUSE 7.6.3 – MASS CONCRETE

7.6.3.2 Temperature requirements for mass concrete

7.6.3.2.1 General
The temperature requirements for mass concrete placements shall comply with Clauses 7.6.3.2.2 to 7.6.3.2.5 unless otherwise specified or approved by the owner.

7.6.3.2.2 Adiabatic Temperature Rise

7.6.3.2.3 Maximum Placing Concrete Temperature

7.6.3.2.4 Maximum Concrete Temperature

7.6.3.2.5 Maximum Concrete Temperature Difference

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CSA A23.1-19, CLAUSE 7.6.3 – MASS CONCRETE

7.6.3.2.4 Maximum concrete temperature

The maximum concrete temperature in mass placements shall not be greater than

- a) 70 °C for non-HVSCM concrete;
- b) 75 °C for HVSCM-2 concrete; and
- c) 85 °C for HVSCM-1 concrete.

Note: The maximum concrete temperature is mainly influenced by the temperature rise of concrete and fresh concrete placing temperature at time of placement. Information and references in Annex T are provided for guidance into the considerations to the applicable maximum concrete temperature for different conditions and concrete properties.

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CSA A23.1-19, CLAUSE 7.6.3 – MASS CONCRETE

7.6.3.2 Temperature requirements for mass concrete

7.6.3.2.1 General

The temperature requirements for mass concrete placements shall comply with Clauses 7.6.3.2.2 to 7.6.3.2.5 unless otherwise specified or approved by the owner.

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CSA A23.1-19, CLAUSE 7.6.3 – MASS CONCRETE

7.6.3.2.5 Maximum concrete temperature difference

7.6.3.2.5.1

The maximum concrete temperature difference in mass placement shall not be greater than 20 °C, specified as a fixed limit, except where higher temperature difference limits are permitted as provided for in Clauses 7.6.3.2.5.2 to 7.6.3.2.5.4.

Note: The maximum temperature difference in mass concrete can be specified in different ways based on the concrete properties and placement attributes. Refer to Annex T for further information and guidance.

7.6.3.2.5.2

A maximum concrete temperature difference fixed limit shall be specified not to exceed 25 °C, when the coefficient of thermal expansion of the concrete is less than 10 millionths/°C, except as provided for in Clause 7.6.3.2.5.4.

7.6.3.2.5.3

An incremental maximum temperature difference limit shall be specified not to exceed 25 °C, except as provided for in Clause 7.6.3.2.5.4.

Note: Refer to Clause 7.4.3.3 for information on incremental temperature difference limits.

7.6.3.2.5.4

A performance based approach based on numerical analysis and modeling with a project specific testing program shall be specified when a maximum concrete temperature difference limit higher than 25 °C is permitted.

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CSA A23.1-19, CLAUSE 7.6.3 – MASS CONCRETE

7.6.3.3 Temperature monitoring

The concrete and ambient temperatures shall be monitored during the thermal control period to determine compliance with temperature requirements in Clause 7.6.3.2. The specific locations for temperature monitoring shall be identified in the thermal control plan. The maximum concrete temperature shall be measured at the interior core of the mass placement where the highest concrete temperature is expected. The temperature differentials between the core and concrete near the surface shall be monitored. The concrete temperature near the surface shall be monitored at a minimum of one representative location between 25 and 75 mm of the concrete surface. Any additional monitoring locations shall be identified in the thermal control plan.

Notes:

- 1) *The maximum temperature at the interior core of the concrete might not be located in the geometrical centre of the mass placement. The concrete temperature near the surface would be typically measured by installing a temperature measuring device tied to a reinforcing bar near the surface.*
- 2) *Monitoring temperature of concrete at or near the surface of a corner or an edge of a mass placement might be necessary for cold weather protection, but it is not a requirement to meet thermal control of mass concrete.*

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CSA A23.1-19, CLAUSE 7.6.3 – MASS CONCRETE

7.6.3.4 Thermal control plan

The contractor shall submit to the owner for approval a thermal control plan to demonstrate that the requirements for controlling and monitoring temperature will be achieved during the thermal control period, including the following information unless otherwise specified or approved by the owner:

- a) dimensions of mass placements;
- b) specified temperature limits;
- c) concrete mix design submittal;
- d) methodology used for thermal analysis and/or modelling;
- e) properties of the concrete;
- f) predicted adiabatic temperature rise of the concrete;
- g) concrete placing temperature considerations;
- h) calculated maximum concrete temperature;
- i) calculated maximum concrete temperature difference;
- j) ambient temperature and weather considerations;
- k) insulation and curing recommendations;
- l) temperature monitoring devices and locations;
- m) requirements to avoid thermal shock (24 h concrete surface temperature drop);
- n) criteria to terminate thermal control;
- o) recommendations to meet temperature limits;
- p) results from thermal analysis and/or modelling;
- q) possible corrective measures;
- r) relevant technical guidelines or references; and
- s) any other information or details such as pre-cooling or active cooling with embedded pipes that might be required to ensure proper implementation of thermal control measures to meet specifications, construction demands, placement attributes, and technical requirements.

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PROJECT IMPLEMENTATION - EXAMPLE

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FOUNDATION STRUCTURE OVERVIEW

Cap/Transfer Plate
Columns
Footings

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OPTIMIZED CONCRETE MIXTURE PROPORTIONS

- **Low Heat**
 - ✓ Lower total cm
 - ✓ Higher fly ash
 - ✓ Better aggregates
 - ✓ Later age strength
- **High Strength**
 - ✓ Lower w/cm ratio
 - ✓ Avoid too high fly ash
- **SCC**
 - ✓ Improved aggregate grading
 - ✓ Balanced paste and mortar
 - ✓ Enhance with admixtures
- **Less Shrinkage**
 - ✓ Reduced water
 - ✓ Enhance with admixtures
- **High MOE**
 - ✓ Good "stiff" coarse aggregate
 - ✓ Qualify with testing

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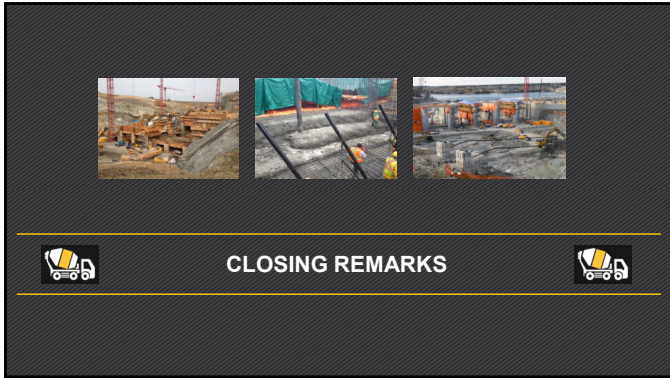
PERFORMANCE BASED THERMAL CONTROL PLAN

PBTCL
Temperature Difference vs. Compressive Strength

Maturity, C-hrs, and Comp St.	Allowable T _{diff}
100	10.0
200	11.0
300	12.0
400	13.0
500	14.0
600	15.0
700	16.0
800	17.0
900	18.0
1000	19.0
1200	20.0
1400	21.0
1600	22.0
1800	23.0
2000	24.0
2500	25.0
3000	26.0
3500	27.0
4000	28.0
4500	29.0
5000	30.0
5500	31.0
6000	32.0
6500	33.0
7000	34.0
7500	35.0
8000	36.0
8500	37.0
9000	38.0
9500	39.0
10000	40.0
11000	41.0
12000	42.0
13000	43.0
14000	44.0
15000	45.0
16000	46.0
17000	47.0
18000	48.0
19000	49.0
20000	50.0

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CLOSING REMARKS

- ✓ **Specifications:**
 - Outdated and need to adapt knowledge to practice
- ✓ **Concrete Mixes:**
 - Optimize concrete mixes to reduce heat of hydration
- ✓ **Temperature Limits:**
 - Know and reduce adiabatic temperature rise
 - Target lower placing concrete temperatures
 - Mitigate maximum "peak" temperature
 - Control maximum temperature difference by testing and developing performance-based limit
- ✓ **Thermal Control:**
 - Follow basic principles to implement thermal control measures to reduce temperature and control temperature changes that could lead to thermal cracking

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QUESTIONS

For more information, contact me at concrete@kiewit.com

THANKS!!!

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